

Structural Engineering and Materials Research Report No. 18/05

SEISMIC PERFORMANCE FACTORS FOR STAND-ALONE TIMBER FRAME STRUCTURES

by

Johnn P. Judd, Ph.D., S.E.

Assistant Professor Department of Civil and Architectural Engineering University of Wyoming

Fernando S. Fonseca, Ph.D., S.E.

Professor
Department of Civil and Environmental Engineering
Brigham Young University

Jacob Leidy

Graduate Research Assistant
Department of Civil and Architectural Engineering
University of Wyoming

Submitted to:

Timber Framers Guild Timber Framing Engineering Council

1106 Harris Ave. Bellingham, Washington 98225

Department of Civil and Architectural Engineering University of Wyoming Laramie, WY 82071

November, 2018

In Equation 5.3.1, R represents the recommended response modification factor, and B_I is a numerical coefficient outlined in Table 18.7-1 of ASCE 7-16 for effective damping, β_I , and period, T. For the analyses conducted, the effective damping was assumed to be 5%, which is typical for most building structures. Based on this effective damping, the numerical coefficient, B_I , was equal to 1.0. Therefore, the deflection amplification factor, C_d , is effectively equal to the response modification factor, R, at a value of 3.0. However, it may be argued that many standalone timber-framed structures do not include many of the non-structural components (mechanical, cladding, etc.) typical in most buildings, such that the effective damping of the structure is probably less ($\leq 2\%$) than the damping used in the analyses. Under these circumstances, the B_I factor would be decreased to 0.8, quantifying the use of a larger deflection amplification factor, $C_d = 3.75$ for design calculations. This C_d value is appropriate for use in design unless the individual structure design justifies a larger system effective damping. Given the uncertainty inherent in predicting the drift of wood systems, however, a C_d value of 4, as a minimum, is recommended.

5.4 Summary of Preliminary Values

This chapter has discussed the performance evaluation of the modeled archetypes and based on the results and analyses, preliminary seismic performance factors for one-story and two-story timber-framed structures with relatively light gravity loading, utilizing pegged kneebraces as the primary seismic force resisting system (SFRS) have been determined. Table 5.4.1 summarizes these values as well as current code values presented in ASCE 7-16.

Table 5.4.1 – Summary of final seismic performance factor recommendations

Factor	Current Code Provision	Recommendation
Response Modification - R	1 ½	3
System Overstrength - $\Omega_{ m o}$	1 ½	3
Deflection Amplification - C _d	1½	4